Nonlinear Vibration and Dynamic Analysis of Functionally Graded Graphene-Reinforced Eccentrically Stiffened Imperfection Composite Cylindrical Shell Panels

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Abstract: This study researches the nonlinear vibration and dynamic response of Functionally Graded Graphene Platelets (FG-GPLs) reinforce eccentrically stiffened imperfection cylindrical Shellpanels. The governing equations are derived in the frame work of the classical Shelltheory coupled with the Von Karman nonlinearity terms. The effective modulus of elasticity for the composite panels and stiffeners is determined with the Halpin-Tsai technique while the rule of mixture is utilized to evaluate the Poisson's ratio and mass density. Galerkin approach coupled with the Runge-Kutta method is employed to solve the nonlinear equation for the dynamic analysis of the Shellpanels. A comparative study is carried out with that available in the open literature to prove the validation of the obtained results. In numerical results, the influence of reinforcing the stiffeners with GPLs, weight fraction, GPLs distribution pattern, geometric parameters of stiffeners, imperfection factor and exciting force on the nonlinear dynamic response of the Shellpanels is analysed in details.

Key words: Nonlinear vibration, dynamic analysis, functionally graded graphene platelets, Shellpanels, eccentrically stiffeners, dynamic loads

INTRODUCTION

Since, the middle of the last decade, Graphene Platelets (GPLs) have been detected (Novoselov *et al.*, 2004) and a wide range of studies have been achieved to discover the properties and performance of this new branch of advanced materials (Spitalsky *et al.*, 2010; Geng *et al.*, 2008; Li *et al.*, 2008). Owing to their excellent and superior mechanical, electrical, thermal and magnetic properties as well as the lower cost production making them stellar choice as enhancement material in the composite structures (Debelak and Lafdi, 2007; Liao *et al.*, 2008; Milani *et al.*, 2013; Wang *et al.*, 2015). As compared with the conventional composites which consume a high volume fraction of the carbon fibre (more than 60%) it's found that by adding a small amount of weight fraction from the carbon filler to the resin matrix (about 0.1-5%) lead to enormous increase in the mechanical properties of the nanocomposite (Putz *et al.*, 2010). In the beginning, most of studies concentrated on the composition and the mechanical characterization of graphene nanocomposite.

Among of them, Rafiee *et al.* (2009) studied experimentally the buckling for composite beams reinforced with graphene and concluded that the adding of 0.1% of graphene leads to increase in the buckling load by 52% in comparison for the case of pure epoxy beam.

Also, Fang *et al.* (2009) fabricated graphene-polystyrene nanocomposite and detected that the adding of 0.9% from graphene leads to increase in the Young's modulus in about of 57%. Wang *et al.* (2008) manufactured new nanocomposite through the adding graphene sheets with various sizes and their research presents that the utilizing for the large size of graphene sheets leads to great in increase in Young's modulus.

A multiscale modelling scheme used by Montazeri and Rafii-Tabar (2011) to study the mechanical characteristic of graphene-reinforced epoxy nanocomposite. Recently, several researches published on the structural behaviour of GPLs-reinforced composite. Chandra *et al.* (2012) analyzed free vibration of graphene-reinforced composite by utilizing the finite element method. Rissanou *et al.* (2015) performed the dynamic and structural analysis for graphene reinforced-nanocomposite by using different sizes of graphene sheets with using of the molecular dynamic simulation. Nowadays, the idea of Functionally Graded Materials (FGMs) to graphene reinforced the resin matrix nanocomposites inserted by Yang and his couthers (Yang *et al.*, 2017) where the GPLs are non-uniformly scattered in the resin matrix. Song *et al.*, (2017a, b) investigated the linear free and forced vibration for the functionally graded material multi-layered graphene reinforced resin matrix composite plates.

They found that the weight fraction and the type of GPLs distribution have a great influence on the frequency and dynamic behaviour of the nanocomposite plates. Also, Song *et al.* (2017a, b) proposed a two-step perturbation technique to investigate the buckling and postbuckling of the functionally graded composite plates reinforced with multi-layer of graphene nanoplatelets under the effect of biaxial compression loads. Shen *et al.*, (2017a, b) presents postbuckling of the composite plates reinforced by functionally graded graphene plates under the effect of thermal loads.

They concluded that the thermal postbuckling can be affected greatly with the functionally graded of graphene sheets. Also, Shen *et al.* (2017a, b) displayed the nonlinear vibration for the laminated composite plates reinforced by the functionally graded graphene sheets under the effect of thermal loads. The nonlinear frequencies determined by employing the two-step perturbation technique. It's found that the distribution kind of reinforcements and the thermal load has considerable effect on the nonlinear frequency. Indeed, the reinforcing the FG-Graphene circular cylindrical shells or cylindrical panels by stiffening members make them capable for carrying heavy dynamic loads in comparative with small addendum weight penalty.

Therefore, the nonlinear dynamic analysis of FG shells reinforced with eccentrically stiffeners received extensive studies from the researchers. Among those, Dung and Nam (2014) investigated the nonlinear dynamic behaviour of FG circular cylindrical shells on elastic foundation and reinforced with eccentrically stiffeners under the effect of external pressure. Bich *et al.* (2012) employed the classical Shelltheory together with the nonlinearity of the Von Karman to analyse the nonlinear dynamic behaviour of eccentrically stiffened FG cylindrical panels. Also, Bich *et al.* (2013) resolved the nonlinear dynamic and static buckling analysis of FG cylindrical thin shells reinforced by eccentrically stiffeners under the influence of axial compression loads. The response of the nonlinear imperfect FG circular cylindrical shells reinforced by eccentrically stiffeners on elastic foundation under the effect of comparison axial loads is studied by Duc and Thang (2014). Also, Duc and Quan (2013) considered the effect of the thermal loads on the nonlinear buckling of FG double curved shallow shells reinforced by external stiffeners on elastic foundation.

Finally, the nonlinear vibrations and dynamic behaviour for the piezoelectric FGM plates reinforced by eccentrically external stiffeners under the effect of mechanical and thermal loads investigated by Duc *et al.*, (2016). It's clear that from the survey for the open literature, there are no publications on the nonlinear vibration and dynamic behaviour of eccentrically stiffened functionally graded graphene platelets-reinforced

composite cylindrical panels. Due to the restrictions in the present industrialization technology, the manufacturing of an uninterruptedly through one dimensions is very tricky if not unattainable.

Therefore, multi-stratums structure containing of a number of individual stratums stacked up through which the weight fraction of the GPLs remains constant through each stratum but keep track of a layer-wise gradual change through the transverse direction is an excellent substantial if the overall number of stratums is adequately massive. The current research aims to search the nonlinear dynamic and vibration analysis of eccentrically stiffened reinforced with functionally graded GPLs-resin matrix composite cylindrical panels under the effect of dynamic load. The modified Halpin-Tsai method is utilized to determine the effective Young's modulus for the composite cylindrical panels and the stiffeners whereas the modified rule of mixture is employed to predict the poisson's ratio mass density.

Theoretical mathematical modelling is based on the classical theory. Galerkin method and Runge-Kutta scheme is used to determine the nonlinear frequencies as well as the dynamic behaviour of the simply supported composite cylindrical panels. A detailed parametric study is carried out to highlight the important insights for the influence of GPLs profile distribution, graphene weight fraction, stiffener geometric parameters, dynamic force and imperfection parameter on the nonlinear frequency as well as the dynamic behaviour of the functionally graded GPLs composite cylindrical panels.

Problem description: A cylindrical Shellpanel consist of a multi-layers GPLs-resin matrix with length a , width b , Radius R and thickness h stiffened with eccentrically stiffeners under the effect of a uniform transverse dynamic load $Q(x, y, t)$ is demonstrated in Fig.1. The cylindrical panel consists of N stratum which has the same thickness and the GPLs uniformly scattered in the resin matrix through individual stratum.

The weight fraction of the GPLs shows a stratum-wise variation to compose a FG structure. With a view of examine the influence of GPLs distribution on the nonlinear frequency and the dynamic response, three kinds of FG material profile of GPLs, i.e., UD, FG_X and FG_O as shown in Fig.2 may be investigated. The weight fraction of the GPLs in the first type UD is uniformly distributed at the same percent fraction across all the stratums. For the two types FG_X and FG_O, the weight fraction varies linearly from stratum to stratum across the panel thickness. The weight fraction decreases from the highest in the top and bottom to the lowest in the middle plane of the panel in the type FG_X whereas in the type FG_O, the case is reversed with the minimum weight fraction on the bottom and top planes and the highest in the middle plane of the panel.

Fig. 1: Geometry of FG_GPLs eccentrically stiffened Shell panel

Fig. 2(a-c): Configuration of different GPLs Shellpanels (a) UD, (b) FG_X and (c) FG_O)

MATERIALS AND METHODS

Modified Halpin-Tsai method is adopted to determine the Young's modulus of the GPLs reinforced resin matrix of the composite panel. By considering GPLs are uniformly scattered in a resin matrix and the effective Young's modulus for the composite panel E can be estimated from the Voigt-Reuses method (Song *et al.*, 2017a, b):

$$E_c = \frac{3}{8}E_L + \frac{5}{8}E_T \quad (1)$$

where, E_L and E_T are the longitudinal and transverse modulus, respectively and can be estimated by Halpin-Tsai method (Song *et al.*, 2017a, b):

$$E_L = \frac{1+\xi_L \eta_L V_{GPL}}{1-\eta_L V_{GPL}} * E_M \quad (2)$$

$$E_T = \frac{1+\xi_w \eta_w V_{GPL}}{1-\eta_w V_{GPL}} * E_M \quad (3)$$

$$\eta_L = \frac{(E_{GPL}/E_M)-1}{(E_{GPL}/E_M)+\xi_L} \quad (4)$$

$$\eta_w = \frac{(E_{GPL}/E_M)-1}{(E_{GPL}/E_M)+\xi_w} \quad (5)$$

$$\xi_L = 2l_{GPL}/h_{GPL} \quad (6)$$

$$\xi_w = 2w_{GPL}/h_{GPL} \quad (7)$$

Setting Eq. 2-7 into Eq. 1 gives:

$$E_c = \frac{3}{8} \frac{(E_{GM} + 2l_{GPL}/h_{GPL}) + 2l_{GPL}/h_{GPL}(E_{GM}-1)V_{GPL}}{E_{GM} + 2l_{GPL}/h_{GPL} - (E_{GM}-1)V_{GPL}} E_M$$

$$\frac{5}{8} \frac{(E_{GM} + 2w_{GPL}/h_{GPL}) + 2w_{GPL}/h_{GPL}(E_{GM}-1)V_{GPL}}{E_{GM} + 2w_{GPL}/h_{GPL} - (E_{GM}-1)V_{GPL}} E_M \quad (8)$$

where, E_{GPL} , l_{GPL} , w_{GPL} and h_{GPL} represent Young's modulus, length, width and thickness for GPLs, respectively while E_M represents Young's modulus for the resin matrix:

$$E_{GM} = E_{GPL}/E_M$$

V_{GPL} is the volume fraction of GPLs and can be estimated from:

$$V_{GPL} = \frac{g_{GPL}}{g_{GPL} + (\rho_{GPL}/\rho_M)(1-g_{GPL})} \quad (9)$$

In which g_{GPL} and ρ_{GPL} represent the weight fraction and mass density for GPLs, respectively while ρ_M is mass density for the resin matrix. The Poisson's ration ν_c and p_c for the composite panel can be estimated from the rule of mixture:

$$\nu_c = \nu_{GPL} V_{GPL} + \nu_M V_M \quad (10)$$

$$\rho_c = \rho_{GPL} V_{GPL} + \rho_M V_M \quad (11)$$

Theoretical model for governing equations: The strains and curvatures at the midst surface of the functionally graded GPLs eccentrically stiffened cylindrical panels in

accordance with the classical Shelltheory with Von Karman nonlinearity are related with the displacement components u, v, w in the x, y, z coordinates (Reddy and Liu, 1985):

$$\begin{aligned}\epsilon_x^o &= \frac{\partial u}{\partial x} + \frac{1}{2} \left(\frac{\partial w}{\partial x} \right)^2, \quad k_x = -\frac{\partial^2 w}{\partial x^2} \\ \epsilon_y^o &= \frac{\partial v}{\partial y} + \frac{1}{R} w + \frac{1}{2} \left(\frac{\partial w}{\partial y} \right)^2, \quad k_y = -\frac{\partial^2 w}{\partial y^2} \\ \gamma_{xy}^o &= \frac{\partial u}{\partial y} + \frac{\partial v}{\partial x} + \frac{\partial w}{\partial x} \frac{\partial w}{\partial y}, \quad k_{xy} = -\frac{\partial^2 w}{\partial x \partial y}\end{aligned}\quad (12)$$

In which R is the radius of the cylindrical panel. The strains at any distance z through the thickness of the panel are:

$$\epsilon_x = \epsilon_x^o + zk_x, \quad \epsilon_y = \epsilon_y^o + zk_y, \quad \gamma_{xy} = \gamma_{xy}^o + 2zk_{xy} \quad (13)$$

For a cylindrical shell, the geometric nonlinear compatibility equation:

$$\frac{\partial^2 \epsilon_x^o}{\partial y^2} + \frac{\partial^2 \epsilon_y^o}{\partial x^2} - \frac{\partial^2 \gamma_{xy}^o}{\partial x \partial y} = \left(\frac{\partial^2 w}{\partial x \partial y} \right)^2 - \frac{\partial^2 w}{\partial x^2} \frac{\partial^2 w}{\partial y^2} - \frac{1}{R} \frac{\partial^2 w}{\partial x^2} \quad (14)$$

The stress components for the Shellat the N th layer can be evaluated from stress-strain relations:

$$\begin{aligned}\sigma_x^{sh(k)} &= \frac{E_c^{(k)}}{1-\nu_c^2} \epsilon_x^{(k)} + \frac{\nu E_c^{(k)}}{1-\nu_c^2} \epsilon_y^{(k)} \\ \sigma_y^{sh(k)} &= \frac{E_c^{(k)}}{1-\nu_c^2} \epsilon_y^{(k)} + \frac{\nu E_c^{(k)}}{1-\nu_c^2} \epsilon_x^{(k)} \\ \sigma_{xy}^{sh(k)} &= \frac{E_c^{(k)}}{2(1-\nu_c)} \gamma_{xy}^{(k)}\end{aligned}\quad (15a)$$

And for the stiffeners (Bich and Long, 2010):

$$\begin{aligned}\sigma_x^s &= E_o \epsilon_x \\ \sigma_y^s &= E_o \epsilon_y\end{aligned}\quad (15b)$$

In which E_o stands for elasticity modulus for the stiffeners and equal the value of elasticity modulus for the top and lower layer of the Shellpanel.

By taking into consideration the effect of stiffeners, the force and moment resultants can be obtained by integrating the stress-strains relations across the thickness of the panel (Najafizadeh *et al.*, 2009):

$$N_x = \left(A_{11} + \frac{E_o A_1}{s_1} \right) \epsilon_x^o + A_{12} \epsilon_y^o - (B_{11} + C_1) k_x - B_{12} k_y$$

$$N_y = A_{12} \epsilon_x^o + \left(A_{22} + \frac{E_o A_2}{s_2} \right) \epsilon_y^o - B_{12} k_x - (B_{22} + C_2) k_y$$

$$N_{xy} = A_{66} \gamma_{xy}^o - 2B_{66} k_{xy}$$

$$M_x = (B_{11} + C_1) \epsilon_x^o + B_{12} \epsilon_y^o - \left(D_{11} + \frac{E_o I_1}{s_1} \right) k_x - D_{12} k_y$$

$$M_y = B_{12} \epsilon_x^o + (B_{22} + C_2) \epsilon_y^o - D_{12} k_x - \left(D_{22} + \frac{E_o I_2}{s_2} \right) k_y$$

$$M_{xy} = B_{66} \gamma_{xy}^o - 2D_{66} \gamma_{xy} \quad (16)$$

In the above relations A_{ij} , B_{ij} and D_{ij} are the Shellstiffness's without stiffeners and represented by:

$$A_{11} = A_{22} = \sum_{k=1}^N \int_{z_k}^{z_{k+1}} \frac{E_c^{(k)}(z)}{1-\nu_c^2} dz, \quad A_{12} = \sum_{k=1}^N \int_{z_k}^{z_{k+1}} \frac{\nu E_c^{(k)}(z)}{1-\nu_c^2} dz$$

$$A_{66} = \sum_{k=1}^N \int_{z_k}^{z_{k+1}} \frac{E_c^{(k)}(z)}{2(1+\nu_c)} dz$$

$$B_{11} = B_{22} = \sum_{k=1}^N \int_{z_k}^{z_{k+1}} \frac{E_c^{(k)}(z)}{2(1+\nu_c^2)} dz, \quad B_{12} = \sum_{k=1}^N \int_{z_k}^{z_{k+1}} \frac{\nu E_c^{(k)}(z)}{(1-\nu_c^2)} dz$$

$$B_{66} = \sum_{k=1}^N \int_{z_k}^{z_{k+1}} \frac{E_c^{(k)}(z)}{2(1+\nu_c)} dz$$

$$D_{11} = D_{22} = \sum_{k=1}^N \int_{z_k}^{z_{k+1}} \frac{E_c^{(k)}(z)}{2(1+\nu_c^2)} z^2 dz$$

$$D_{12} = \sum_{k=1}^N \int_{z_k}^{z_{k+1}} \frac{\nu E_c^{(k)}(z)}{2(1-\nu_c^2)} z^2 dz, \quad D_{66} = \sum_{k=1}^N \int_{z_k}^{z_{k+1}} \frac{E_c^{(k)}(z)}{2(1+\nu_c)} z^2 dz \quad (17)$$

Also, the relations for stiffeners geometric parameters can be represented by:

$$I_1 = \frac{d_1 h_1^3}{12} + A_1 z_1^2, \quad I_2 = \frac{d_2 h_2^3}{12} + A_2 z_2^2$$

$$C_1 = \frac{E_c A_1 z_1}{s_1}, \quad C_2 = \frac{E_c A_2 z_2}{s_2}$$

$$z_1 = \frac{h_1 + h}{2}, \quad z_2 = \frac{h_2 + h}{2} \quad (18)$$

The strains in terms of the force resultants can be obtained from Eq. 16:

$$\begin{aligned}\varepsilon_x^o &= E_{22}N_x - E_{12}N_y + G_{11}k_x + G_{12}k_y \\ \varepsilon_y^o &= E_{11}N_y - E_{12}N_x + G_{21}k_x + G_{22}k_y \\ \gamma_{xy}^o &= E_{66} + 2G_{66}k_{xy}\end{aligned}\quad (19)$$

In which:

$$E_{11} = \frac{1}{\Lambda} \left(A_{11} + \frac{E_c A_1}{s_1} \right), E_{22} = \frac{1}{\Lambda} \left(A_{22} + \frac{E_c A_2}{s_2} \right)$$

$$E_{12} = \frac{A_{12}}{\Lambda}, E_{66} = \frac{1}{A_{66}}, \Lambda = \left(A_{11} + \frac{E_c A_1}{s_1} \right) \left(A_{22} + \frac{E_c A_2}{s_2} \right) - A_{12}^2$$

$$G_{11} = E_{22}(B_{11} + C_1) - E_{12}B_{12}, G_{22} = E_{11}(B_{22} + C_2) - E_{12}B_{12}$$

$$G_{12} = E_{22}B_{12} - E_{12}(B_{22} + C_2), G_{21} = E_{11}B_{12} - E_{12}(B_{11} + C_1) \quad (20)$$

$$G_{66} = \frac{B_{66}}{A_{66}}$$

By substituting Eq. 18 into Eq. 15 gives:

$$\begin{aligned}M_x &= G_{11}N_x + G_{21}N_y - F_{11}k_x - F_{12}k_y \\ M_y &= G_{12}N_x + G_{22}N_y - F_{21}k_x - F_{22}k_y \\ M_{xy} &= G_{66}N_{xy} - 2F_{66}N_{xy}\end{aligned}\quad (21)$$

In which:

$$F_{11} = D_{11} + \frac{E_c I_1}{s_1} - (B_{11} + C_1) G_{11} - G_{12}G_{21}$$

$$F_{22} = D_{22} + \frac{E_c I_2}{s_2} - (B_{22} + C_2) G_{22} - B_{12}G_{22}$$

$$F_{12} = D_{12} - (B_{11} + C_1) G_{12} - B_{12}G_{22} \quad (22)$$

$$F_{21} = D_{12} - B_{12} G_{11} - (B_{22} + C_2) G_{21}$$

$$F_{66} = D_{66} - B_{66}G_{66}$$

In accordance with the classical Shell theory, the nonlinear equations of motion are:

$$\frac{\partial N_x}{\partial x} + \frac{\partial N_{xy}}{\partial y} = 0$$

$$\frac{\partial N_{xy}}{\partial x} + \frac{\partial N_y}{\partial y} = 0$$

$$\begin{aligned}\frac{\partial^2 M_x}{\partial x^2} + 2 \frac{\partial^2 M_{xy}}{\partial x \partial y} + \frac{\partial^2 M_y}{\partial y^2} + N_x \frac{\partial^2 w}{\partial x^2} + \\ 2N_{xy} \frac{\partial^2 w}{\partial x \partial y} + N_y \frac{\partial^2 w}{\partial y^2} + \frac{1}{R} N_y + q_o = I_1^* \frac{\partial^2 w}{\partial t^2}\end{aligned}\quad (23)$$

$$I_1^* = \sum_{k=1}^N \int_{z_k}^{z_{k+1}} \rho_c(z) dz + \rho_o \left(\frac{A_1}{s_1} + \frac{A_2}{s_2} \right)$$

In which:

Where:

ρ_c : The mass density for the composite panel

ρ_o : The mass density for the stiffeners and takes the same value of mass density for the top layer of the composite panel

The stress function:

$$N_x = \frac{\partial^2 f}{\partial y^2}, N_y = \frac{\partial^2 f}{\partial x^2}, N_{xy} = -\frac{\partial^2 f}{\partial x \partial y} \quad (24)$$

The inserting of Eq. 19 into Eq. 14 and 21 into Eq. 23 with taken into consideration Eq. 12 and 24 gives a system of nonlinear equations:

$$\begin{aligned}E_{11} \frac{\partial^4 f}{\partial x^4} + (E_{66} - 2E_{12}) \frac{\partial^4 f}{\partial x^2 \partial y^2} + E_{22} \frac{\partial^4 f}{\partial y^4} + G_{21} \frac{\partial^4 w}{\partial x^4} + \\ (G_{11} + G_{22} - 2G_{66}) \frac{\partial^4 w}{\partial x^2 \partial y^2} + G_{12} \frac{\partial^4 w}{\partial y^4} + \frac{1}{R} \frac{\partial^4 w}{\partial x^2} = \\ \left(\frac{\partial^2 w}{\partial x \partial y} \right)^2 \frac{\partial^2 w}{\partial x^2 \partial y^2}\end{aligned}\quad (25)$$

$$\begin{aligned}I_1^* \frac{\partial^2 w}{\partial t^2} + F_{11} \frac{\partial^4 w}{\partial x^4} + (F_{12} + F_{21} + 4F_{66}) \frac{\partial^4 w}{\partial x^2 \partial y^2} + F_{22} \frac{\partial^4 w}{\partial y^4} - G_{21} \frac{\partial^4 f}{\partial x^4} - \\ (G_{11} + G_{22} - 2G_{66}) \frac{\partial^4 f}{\partial x^2 \partial y^2} - G_{12} \frac{\partial^4 f}{\partial y^4} - \frac{1}{R} \frac{\partial^2 f}{\partial x^2} - \frac{\partial^2 f}{\partial y^2} \frac{\partial^2 w}{\partial x^2} + \\ 2 \frac{\partial^2 f}{\partial x \partial y} \frac{\partial^2 w}{\partial x \partial y} - \frac{\partial^2 f}{\partial x^2} \frac{\partial^2 w}{\partial y^2} = q_o\end{aligned}\quad (26)$$

For taking into consideration, the initial imperfection for the FG-GPLs cylindrical panels, w_o represent a known very small imperfection and as in the Volmir's Model (Shen, 2001) for an imperfect cylindrical panels, the equation of motion for an imperfect FG-GPLs stiffened cylindrical panels becomes:

$$\begin{aligned}E_{11} \frac{\partial^4 f}{\partial x^4} + (E_{66} - 2F_{12}) \frac{\partial^4 f}{\partial x^2 \partial y^2} + E_{22} \frac{\partial^4 f}{\partial y^4} + G_{21} \frac{\partial^4 (w - w_o)}{\partial x^4} + \\ (G_{11} + G_{22} - 2G_{66}) \frac{\partial^4 (w - w_o)}{\partial x^2 \partial y^2} + G_{12} \frac{\partial^4 (w - w_o)}{\partial y^4} + \frac{1}{R} \\ \frac{\partial^2 (w - w_o)}{\partial x^2} = \left[\left(\frac{\partial^2 w}{\partial x \partial y} \right)^2 \frac{\partial^2 w}{\partial x^2 \partial y^2} \right] - \left[\left(\frac{\partial^2 w_o}{\partial x \partial y} \right)^2 \frac{\partial^2 w_o}{\partial x^2 \partial y^2} \right]\end{aligned}\quad (27)$$

$$\begin{aligned} & I_1 \frac{\partial^2 w}{\partial t^2} + F_{11} \frac{\partial^4 (w-w_0)}{\partial x^4} + (F_{12} + F_{21} + 4F_{66}) \frac{\partial^4 (w-w_0)}{\partial x^2 \partial y^2} + \\ & F_{22} \frac{\partial^4 (w-w_0)}{\partial y^4} - G_{21} \frac{\partial^4 f}{\partial x^4} - (G_{11} + G_{22} - 2G_{66}) \frac{\partial^4 f}{\partial x^2 \partial y^2} - G_{12} \frac{\partial^4 f}{\partial y^4} \quad (28) \\ & \frac{1}{R} \frac{\partial^2 f}{\partial x^2} \frac{\partial^2 f}{\partial y^2} \frac{\partial^2 w}{\partial x^2} + 2 \frac{\partial^2 f}{\partial x \partial y} \frac{\partial^2 w}{\partial x \partial y} \frac{\partial^2 f}{\partial x^2} \frac{\partial^2 w}{\partial y^2} = q_0 \end{aligned}$$

In which w represent the deflection of the cylindrical panel. Equation 27 and 28 are nonlinear equations and can be employed to study the vibration and dynamic behaviour for imperfection FG-GPLs eccentrically stiffened cylindrical panels.

Problem solution: By assuming an imperfect FG-GPLs eccentrically stiffened cylindrical panel is simply supported under the effect of uniformly distributed pressure q_0 . So, for the present case study, the boundary conditions:

$$\begin{aligned} w=0, M_x=0, N_x=0, N_{xy}=0 \text{ at } x=0, x=a \\ w=0, M_y=0, N_y=0, N_{xy}=0 \text{ at } y=0, y=a \end{aligned} \quad (29)$$

The above boundary conditions can be satisfied if the solution for the panel deflection in the form as:

$$w = w(t) \sin \frac{m\pi x}{a} \sin \frac{n\pi y}{b} \quad (30)$$

In which $w(t)$ is function of time and represent the amplitude, m, n represent the half wave number in the longitudinal and circumferential directions. For the case of initial imperfection, it's consider identically to the solution for the panel deflection:

$$w_0 = w_0 \sin \frac{m\pi x}{a} \sin \frac{n\pi y}{b} \quad (31)$$

By solving Eq. 30 and 31 with Eq. 14 and by defining the stress function as:

$$\begin{aligned} f(x, y, t) = A_1(t) \cos \frac{2m\pi x}{a} + A_2(t) \cos \frac{2n\pi y}{b} - \\ A_3(t) \sin \frac{m\pi x}{a} \sin \frac{n\pi y}{b} \end{aligned} \quad (32)$$

In which:

$$\begin{aligned} A_1 = \frac{n^2(a/b)^2 w^2}{32m^2 E_{11}}, A_2 = \frac{m^2 w^2}{32n^2(a/b)^2 E_{22}}, A_3 = \\ \frac{[G_{21}m^4 + (G_{11} + G_{22} - 2G_{66})m^2n^2(a/b)^2 + G_{12}n^4(a/b)^4 - (a^2/\pi^2)(1/R)m^2]}{E_{11}m^4 + (E_{66} - 2E_{12})m^2n^2(a/b)^2 + E_{22}n^4(a/b)^4} \end{aligned} \quad (33)$$

By substituting the relations (30-32) into Eq. 26 and then by applying Galerkin in approach, it gives:

$$\begin{aligned} a_1 w'' + \left(a_2 + \frac{a_3^2}{a_4}\right)(w-w_0) + \frac{32mn(a/b)^2}{3\pi} \frac{a_3}{a_4} (w-w_0)w + \\ a_5(w^2-w_0^2) + a_6(w^2-w_0^2)w - \frac{16a^4 q_0}{mn\pi^6} = 0 \end{aligned} \quad (34)$$

In which:

$$\begin{aligned} a_1 = \frac{a^4}{\pi^4} I_1, a_2 = m^4 F_{11} + (F_{12} + F_{21} + 4F_{66})m^2n^2(a/b)^2 + F_{22}n^4(a/b)^4 \\ a_3 = G_{21}m^4 + (G_{11} + G_{22} - 2G_{66})m^2n^2(a/b)^2 + G_{12}n^4(a/b)^4 - \frac{a^2}{\pi^2} \frac{1}{R}m^2 \\ a_4 = E_{11}m^4 + (E_{66} - 2E_{12})m^2n^2(a/b)^2 + E_{22}n^4(a/b)^4 \\ a_5 = \frac{8mn(a/b)^2}{3\pi^2} \left(\frac{G_{21}}{E_{11}} + \frac{G_{12}}{E_{22}}\right) - \left(\frac{8a^2}{6\pi^4 mn} \frac{n^2(a/b)^2}{E_{11}} \frac{1}{R}\right) \\ a_6 = \frac{1}{16} \left(\frac{m^4}{E_{22}} + \frac{n^4(a/b)^4}{E_{11}}\right) \end{aligned} \quad (35)$$

By considering the uniform distributed load $q_0 = Q \sin \Omega t$ Eq. 34 becomes:

$$\begin{aligned} a_1 w'' + \left(a_2 + \frac{a_3^2}{a_4}\right)(w-w_0) + \frac{32mn(a/b)^2}{3\pi} \frac{a_3}{a_4} (w-w_0) \\ w + a_5(w^2-w_0^2) + a_6(w^2-w_0^2)w = \frac{16a^4}{mn\pi^6} Q \sin \Omega t \end{aligned} \quad (36)$$

Thus, the nonlinear dynamic behaviour for the FG-GPLs imperfection stiffened cylindrical panels can be determined by resolving the above equation in combination with the initial conditions to be considered as $w(0) = 0, w'(0) = 0$, by employing the Runge-Kutta method. For linear and free vibration, Eq. 36 becomes:

$$a_1 w'' + \left(a_2 + \frac{a_3^2}{a_4}\right)(w-w_0) = 0 \quad (37)$$

From the above equation, the fundamental frequencies for the natural vibration of FG-GPLs imperfection stiffened panel can be obtained as:

$$\omega_L = \sqrt{\frac{1}{a_1} \left(a_2 + \frac{a_3^2}{a_4}\right)} \quad (38)$$

Also, for nonlinear free vibration, Eq. 36 with perfect panel can be written as:

$$w'' + n_1 w + n_1 w^2 + n_3 w^3 = 0 \quad (39)$$

$$n_1 = \frac{1}{a_1} \left(a_2 + \frac{a_3^2}{a_4}\right), n_2 = \frac{1}{a_1} \left(\frac{32mn(a/b)^2}{3\pi^2} \frac{a_3}{a_4} + a_5\right), n_3 = \frac{a_6}{a_1} \quad (40)$$

Let $w(t) = X \sin \Omega t$ and by applying Galerkin method to Eq. 39, the frequency-amplitude relation can be determined:

$$\frac{\omega_{NL}}{\omega_L} = \left(1 + \frac{8n_2}{3\pi\omega_L^2} X + \frac{3n_3}{4\omega_L^2} X^2 \right)^{\frac{1}{2}} \quad (41)$$

Where:

ω_{NL} : The nonlinear vibration frequency

X : The amplitude of nonlinear vibration

RESULTS AND DISCUSSION

Validation of the current case study: In this portion, to prove validation the present case approach, the fundamental frequency change with the weight fraction of GPLs for multi-stratums FG-GPLs reinforcing epoxy matrix plates are compared in Fig. 3 with the results obtained in Song *et al.* (2017a, b). The variations in the fundamental frequencies are represented for different GPLs distribution patterns. This comparison reveals that the outcomes from the current approach are in a good acceptance with the published results.

Vibration results: In order to explain the suggested analysis for multi-stratums FG-GPLs reinforced with external stiffeners thin cylindrical panels ($a \times b \times h = 1.5 \times 1.5 \times 0.008$ m), the dimensions and material properties for GPLs and resin matrix are:

$$\begin{aligned} l_{GPL} &= 2.5 \mu\text{m}, \quad w_{GPL} = 1.5 \mu\text{m}, \quad h_{GPL} = 1.5 \text{ nm}, \\ \rho_{GPL} &= 1.06 \text{ g cm}^{-3}, \quad E_{GPL} = 1.01 \text{ Tpa}, \quad \rho_m = 1.2 \text{ g cm}^{-3}, \\ E_M &= 3.0 \text{ Gpa}, \quad \nu_m = 0.34, \quad \nu_{GPL} = 0.186 \end{aligned}$$

In addition, the dimensions for geometric parameters of stiffeners:

$$h_1 = h_2 = 0.015 \text{ m}, \quad s_1 = s_2 = 0.015 \text{ m}, \quad d_1 = d_2 = 0.04 \text{ m}$$

Results of natural vibrations: The results taken out in Table 1 exhibit that the influence of stiffeners on the fundamental frequencies for the natural vibration (ω_L) is significant. It's clearly that the fundamental frequencies for FG-GPLs panels reinforcing with stiffeners or without stiffeners to be relied mainly on the weight fraction and distribution style of GPLs. It is evident that the percentage increase in ω_L is 105.9% when the weight fraction of GPLs increase from 1-5% in the case of FG_X distribution pattern. Also in the case of FG_X and FG_O distribution pattern, the percentage increase in the natural frequencies is 47.4%.

Results of nonlinear frequency-amplitude free vibration: Figure 4 displays the effect of weight fraction on the nonlinear frequency for different GPLs distribution

Fig. 3: Comparison of relative frequency change-weight fraction with results published by Song *et al.* (2017a, b) for different distribution patterns

Fig. 4: Effect of weight fraction (wt.) and distribution patterns on nonlinear frequency-amplitude relation

patterns. As predicted, the nonlinear vibration frequencies increase with the increasing in the weight fraction. Also, the nonlinear frequencies for (FG_X) distribution pattern are greater than for (UD) uniform distribution style. Figure 5 shows the effect of stiffeners thickness on the nonlinear frequencies with (FG_X) distribution pattern. It is obviously that the small increasing in the stiffener thickness lead to considerable increasing in the nonlinear frequencies. Figure 6 and 7 demonstrates the effect of spacing (s_1) and height (h_1) of stiffeners on the nonlinear frequency-amplitude relation, respectively. It's clearly that the effect of parameter (s_1) on the nonlinear frequencies is greater than the influence of parameter (h_1) Table 1.

Results of nonlinear dynamic behaviour: In this part, the effects of the GPLs distribution pattern, weight fraction of GPLs, exciting force, imperfection and geometric parameters of stiffeners on the nonlinear dynamic response of the FG-GPLs Shell panel will be

Table 1: The natural frequencies of (rad/s) of FG-GPLs cylindrical panels

R (m)	GPLs distribution pattern	wt. %	Stiffened (m, n)	Unstiffened (m, n)
1.5	FG_X	1	1079.4 (1,2)	460.648 (1,3)
		3	1746.7 (1,2)	745.731 (1,3)
		5	2223.0 (1,2)	948.924 (1,3)
	UD	1	980.1095 (1,2)	416.725 (1,3)
		3	1561.9 (1,2)	664.0535 (1,3)
		5	1980.6 (1,2)	842.0113 (1,3)
	FG_O	1	849.29 (1,2)	367.6279 (1,3)
		3	1299.9 (1,2)	570.5389 (1,3)
		5	1629.8 (1,2)	718.7269 (1,3)
3.0	FG_X	1	1030.3 (1,2)	298.603 (1,2)
		3	1670.8 (1,2)	480.535 (1,2)
		5	2127.4 (1,2)	610.855 (1,2)
	UD	1	919.65 (1,2)	281.995 (1,2)
		3	1465.5 (1,2)	449.554 (1,2)
		5	1858.3 (1,2)	570.2707 (1,2)
	FG_O	1	766.1391 (1,2)	264.39 (1,2)
		3	1155.9 (1,2)	416.249 (1,2)
		5	1443.0 (1,2)	526.471 (1,2)
5.0	FG_X	1	430.469 (1,1)	241.549 (1,2)
		3	693.664 (1,1)	390.519 (1,2)
		5	882.0346 (1,1)	496.815 (1,2)
	UD	1	403.202 (1,1)	220.704 (1,2)
		3	642.6539 (1,1)	351.730 (1,2)
		5	815.0629 (1,1)	446.0347 (1,2)
	FG_O	1	372.294 (1,1)	197.697 (1,2)
		3	583.0575 (1,1)	307.98 (1,2)
		5	736.24 (1,1)	388.398 (1,2)

Fig. 5: Effect of stiffeners thickness on frequency-amplitude relation

consider. Figure 7 illustrates the influence of weight fraction of graphene on the nonlinear dynamic response of FG-GPLs Shellpanel.

It can observe that the amplitude of nonlinear vibration of the panel increases with the increasing in the weight fraction. The effect of distribution pattern for GPLs on the vibration amplitude of FG-GPLs Shellis demonstrates in Fig. 8. It's evident that the vibration amplitude for (FG_X) distribution pattern is the lower and (FG_O) is the higher while the amplitude for the Uniform Distribution pattern (UD) lies in between. Figure 9

Fig. 6: Effect of stiffeners spacing on frequency-amplitude relation

Fig. 7: Effect of weight fraction (wt.) on the dynamic response of FG-GPLs cylindrical panel

Fig. 8: Effect of distribution pattern on the dynamic response of FG-GPLs cylindrical panel

Fig. 11: Effect of exciting force on the dynamic response of FG-GPLs cylindrical panel

Fig. 9: Effect of imperfection parameter on the dynamic response of FG-GPLs cylindrical panel

Fig. 12: Effect of stiffeners thickness on the dynamic response of FG-GPLs cylindrical panel

Fig. 10: Effect of stiffeners on the dynamic response of FG-GPLs cylindrical panel

Fig. 13: Effect of stiffeners spacing on the dynamic response of FG-GPLs cylindrical panel

characterizes the nonlinear dynamic response of eccentrically stiffened FG-GPLs Shellpanel under the effect of the imperfection parameter. It's clear that the vibration amplitude is greatly affected with the imperfection parameter. Figure 10 displays the influences of stiffeners on the vibration response for the Shellpanels reinforced with FG graphene sheets.

The outcome displays that the stiffeners robustly decrease the vibration response of the Shell panel. Figure 11 points to the influence of the exciting force on

the vibration amplitude for the Shellpanels. It can be seen that the FG-GPLs Shellpanel variation amplitudes increase when the exciting force increase. Figure 12-14 indicate the effect of geometric parameters of stiffeners on the dynamic response of FG-GPLs Shell panels. From Fig. 13 the vibration amplitude decreases when increasing stiffener thickness (d_1). Figure 14 characterizes the effect

Fig. 14: Effect of stiffeners height on the dynamic response

of stiffener spacing on the nonlinear dynamic behaviour for the FG-GPLs Shellpanels and the panel fluctuates weakly when the factor (s_1) is decreased. Finally, Fig. 14 shows the influence of stiffener height the vibration response of the shell. It's evident that the increasing the parameter (h_1) leads to decreasing the vibration amplitude.

CONCLUSION

A classical Shelltheory coupled with Von Karman nonlinear terms has been employed in the formulization of the governing equations of FG graphene-reinforced composite eccentrically stiffened cylindrical Shellpanels. Galerkin solution based technique is utilized to determine the fundamental frequencies, nonlinear frequencies as well as the dynamic response for simply supported cylindrical panels under the effect of dynamic loading. The impact of graphene distribution style, weight fraction, stiffeners and geometric parameters of stiffeners, imperfection factor and the exciting force on the nonlinear dynamic behaviour of the Shellpanel has been investigated. It is that reinforcing the stiffeners with graphene sheets plays an important role in increasing the nonlinear frequencies and decreasing the dynamic deflections an addendum a very small amount of graphene sheets able to considerable increasing nonlinear frequencies and decreasing the vibration amplitude graphene distribution pattern (i.e., FG_X) is the effective way to enhance the Shellpanels.

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